

# Delegated Agency Training September 23, 2024 by Raymond Morgan, PE

Hydraulic Linear Loading Rates  
Calculations

Buoyancy Calculations

Cesspools vs Earth Pit Privy



Clean Air, Safe Water,  
Healthy Land for Everyone

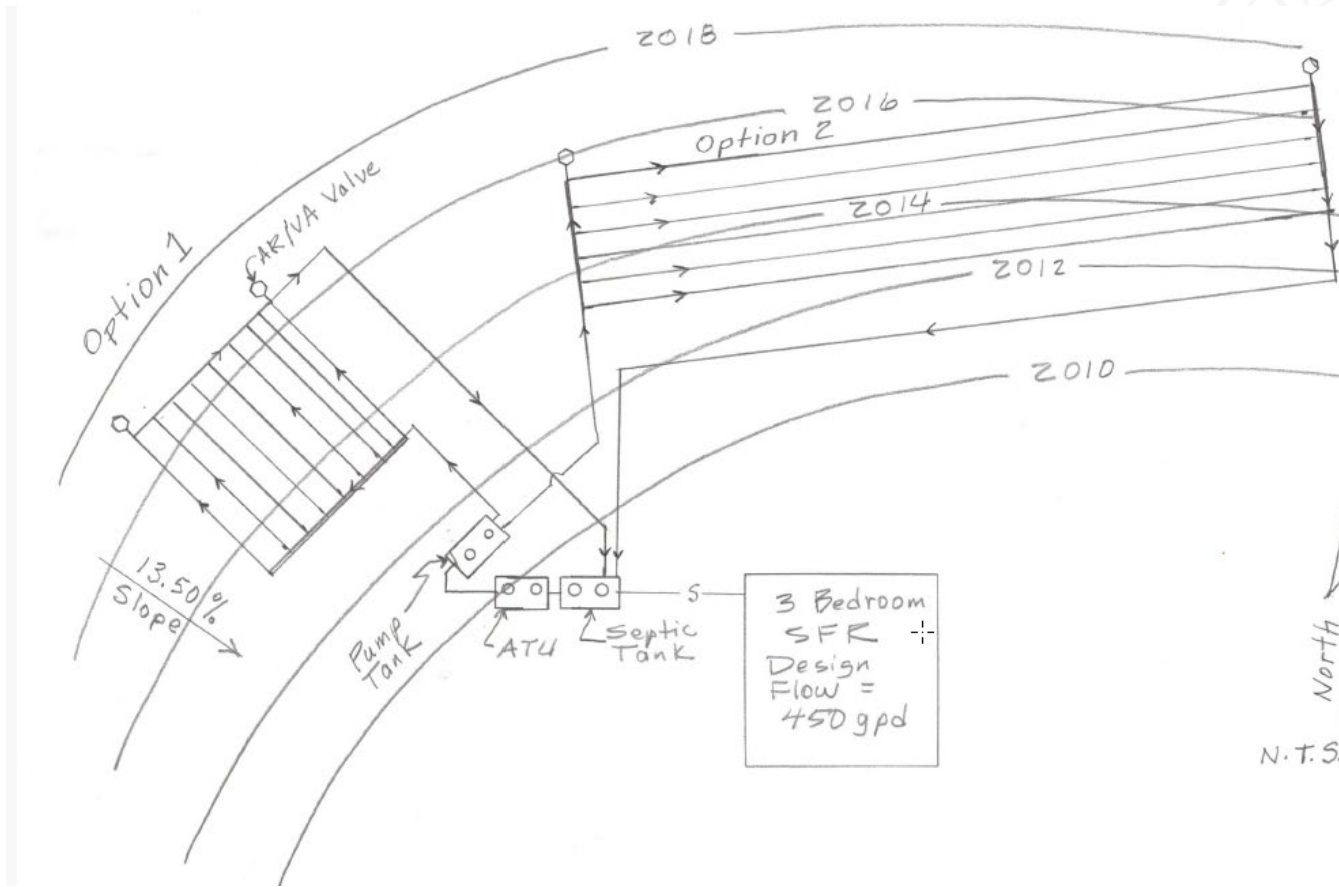


- **R18-9-A312. Facility Design for Type 4 On-site Wastewater Treatment Facilities**
- **A. General design requirements. An applicant shall ensure that the person designing an on-site wastewater treatment facility:**
  - 1. Signs the design documents submitted as part of the Notice of Intent to Discharge to obtain a Construction Authorization, including plans, specifications, drawings, reports, and calculations; and
  - 2. Locates and designs the on-site wastewater treatment facility project using good design judgment and relies on appropriate design methods and calculations.

- R18-9-A312.B.4 For on-site wastewater treatment facilities permitted under R18-9-E303 through R18-9-E323, apply the following design requirements, as applicable:
  - a. Include the power source and power components in construction drawings if electricity or another type of power is necessary for facility operation;
  - b. If a hydraulic analysis is required under subsection (E), perform the analysis based on the location and dimensions of the bottom and sidewall surfaces of the disposal works that are identified in the design documentation;

- R18-9-A312.E.2.b Include a **hydraulic analysis** with the Notice of Intent to Discharge, based on the dimensions of the absorption surfaces specified in R18-9-A312(B)(4)(b), showing that the **soil is sufficiently permeable to conduct wastewater downward and laterally without surfacing** for the site conditions at the disposal works.

# Hydraulic Linear Loading Rate



## ■ Information for the Onsite Wastewater Treatment Facility Design (OWTF):

- Three-bedroom single family residence
- Design Flow = 3 bedrooms x 150 gpd/bedroom = 450 gpd
- SAR – 0.60 gpd/sf
- Soil Texture - Sandy Loam (SL)
- Structure – Moderate
- Soil Depth – 4 feet
- SAR<sub>a</sub> – 1.20 gpd/sf
- BOD<sub>5</sub> – 15 mg/l
- TSS – 15 mg/l
- Allowable Hydraulic Linear Loading Rate (AHLLR) – 7 gpd/lf (from Tyler's Table 1)
- Dripline – Drip Heads at 2' intervals
- Drip Emitter Flow Rate – 0.60 gph



# Hydraulic Linear Loading Rate

## 2005 Soil Absorption Rate Adjustment Calculation -

**Enter Data as Follows:**

BOD (Effluent Value)	TSS (Effluent Value)	Unadjusted SAR (From Site Investigation)	Design Flow (gpd)		
15	15	0.60	450		

## 2005 Soil Absorption Rate Adjustment Formula

BOD (Effluent Value)	TSS (Effluent Value)	Unadjusted SAR (From Site Investigation)	Adjusted SAR (Calculated Value)	Unadjusted Disposal Field Area (sf)	Adjusted Disposal Field Area (sf)
15	15	0.60	1.20	750	373

# Hydraulic Linear Loading Rate

Soil characteristics				Hydraulic linear loading rate, gal/d/ft												
				Infiltration loading rate, gal/da/ft <sup>2</sup>				Slope								
								0-4%			5-9%			>10%		
Texture	Structure		>30 mg/L	<30 mg/L	Infiltration distance, inch			Infiltration distance, inch			Infiltration distance, inch					
	Shap	Grad			8-12	12-24	24-48	8-	12-	24-	8-	12-	24-			
COS, S, LCOS, LS	--	OSG	0.8	1.6	4.0	5.0	6.0	5.0	6.0	7.0	6.0	7.0	8.0			
FS,VFS,LFS,LVFS	--	OSG	0.4	1.0	3.5	4.5	5.5	4.0	5.0	6.0	5.0	6.0	7.0			
CSL, SL	--	OM	0.2	0.6	3.0	3.5	4.0	3.6	4.1	4.6	5.0	6.0	7.0			
	PL	1	0.2	0.5	3.0	3.5	4.0	3.6	4.1	4.6	4.0	5.0	6.0			
		2,3	0.0	0.0	-	-	-	-	-	-	-	-	-			
	PR/B K/G R	1	0.4	0.7	3.5	4.5	5.5	4.0	5.0	6.0	5.0	6.0	7.0			
2,3		0.6	1.0	3.5	4.5	5.5	4.0	5.0	6.0	5.0	6.0	7.0				
FSL, VFSL	--	OM	0.2	0.5	2.0	2.3	2.6	2.4	2.7	3.0	2.7	3.2	3.7			
	PL	1,2,3	0.0	0.0	-	-	-	-	-	-	-	-	-			
		1	0.2	0.6	3.0	3.5	4.0	3.3	3.8	4.3	3.6	4.1	4.6			
	PR/B K/G	2,3	0.4	0.8	3.3	3.8	4.3	3.6	4.1	4.6	3.9	4.4	4.9			
L		--	OM	0.2	0.5	2.0	2.3	2.6	2.4	2.7	3.0	2.7	3.2	3.7		
	PL	1,2,3	0.0	0.0	-	-	-	-	-	-	-	-	-			
		1	0.4	0.6	3.0	3.5	4.0	3.3	3.8	4.3	3.6	4.1	4.6			
	PR/B K/G R	2,3	0.6	0.8	3.3	3.8	4.3	3.6	4.1	4.6	3.9	4.4	4.9			
SIL		--	OM	0.0	0.2	2.0	2.5	3.0	2.2	2.7	3.2	2.4	2.9	3.4		
	PL	1,2,3	0.0	0.0	-	-	-	-	-	-	-	-	-			
		1	0.4	0.6	2.4	2.7	3.0	2.7	3.0	3.3	3.0	3.5	4.0			
	PR/B K/G	2,3	0.6	0.8	2.7	3.0	3.3	3.0	3.5	4.0	3.3	3.8	4.3			
SCL,CL SICL		--	OM	0.0	0.0	-	-	-	-	-	-	-	-	-		
	PL	1,2,3	0.0	0.0	-	-	-	-	-	-	-	-	-			
		1	0.2	0.3	2.0	2.5	3.0	2.2	2.7	3.2	2.4	2.9	3.4			
	PR/B K/G	2,3	0.4	0.6	2.4	2.9	3.4	2.7	3.0	3.3	3.0	3.5	4.0			
SC, C, SIC		--	OM	0.0	0.0	-	-	-	-	-	-	-	-	-		
	PL	1,2,3	0.0	0.0	-	-	-	-	-	-	-	-	-			
		1	0.0	0.0	-	-	-	-	-	-	-	-	-			
	PR/B K/G R	2,3	0.2	0.3	2.0	2.5	3.0	2.2	2.7	3.2	2.4	2.9	3.4			

Hydraulic Wastewater Loading Rates to Soil – Table 1 by E. Jerry Tyler



- Calculations – Disposal Area on 13.5% Slope
  - Disposal Area Needed (DAN) = Design Flow/SAR<sub>a</sub>
  - DAN = 450 gpd/1.20 gpd/sf = 375 sf
  - DAN Dimensions (option 1) =  $(375)^{0.50} = 20' \times 20'$ 
    - Actual HLLR = 450 gpd/20' = 22.5 gpd/lf > 7 gpd/lf
  - DAN Dimensions (option 2) = 6' x 65'
    - Length = Design Flow/AHLLR = 450 gpd/7 gpd/lf = 64.3'
    - Width = DAN/Length = 375 sf / 64.3' = 5.8'

- Calculations – Disposal Area on Level Building Pad
  - Disposal Area Needed = Design Flow/SAR<sub>a</sub>
  - DAN = 450 gpd/1.20 gpd/sf = 375 sf
  - DAN Dimensions (option 1) =  $(375)^{0.50} = 20' \times 20'$ 
    - Actual HLLR = 450 gpd/20' = 22.5 gpd/lf > 5.5 gpd/lf
  - DAN Dimensions (option 2) = 6' x 82'
    - Length = Design Flow/AHLLR = 450 gpd/5.5 gpd/lf = 81.8'
    - Width = DAN/Length = 375 sf / 81.8' = 4.58'

- Calculations – Number of Drip Emitters & Dispersal Flow Rate
  - Number of Emitters = (# of rows (3) x 82') / 2' (emitter spacing) = 123
  - Hourly Design Flow (HDF) = 450 gpd/24 hpd = 18.75 gph
  - Hourly Dispersal Rate (HDR) = emitter flow rate (gph/emitter) x # of emitters = 0.60 gph x 123 emitters = 73.80 gph
- Dosing Time Ratio = HDF (gph)/ HDR (gph) = .25, therefore only ¼ of the hour is needed to disperse the effluent Design Flow

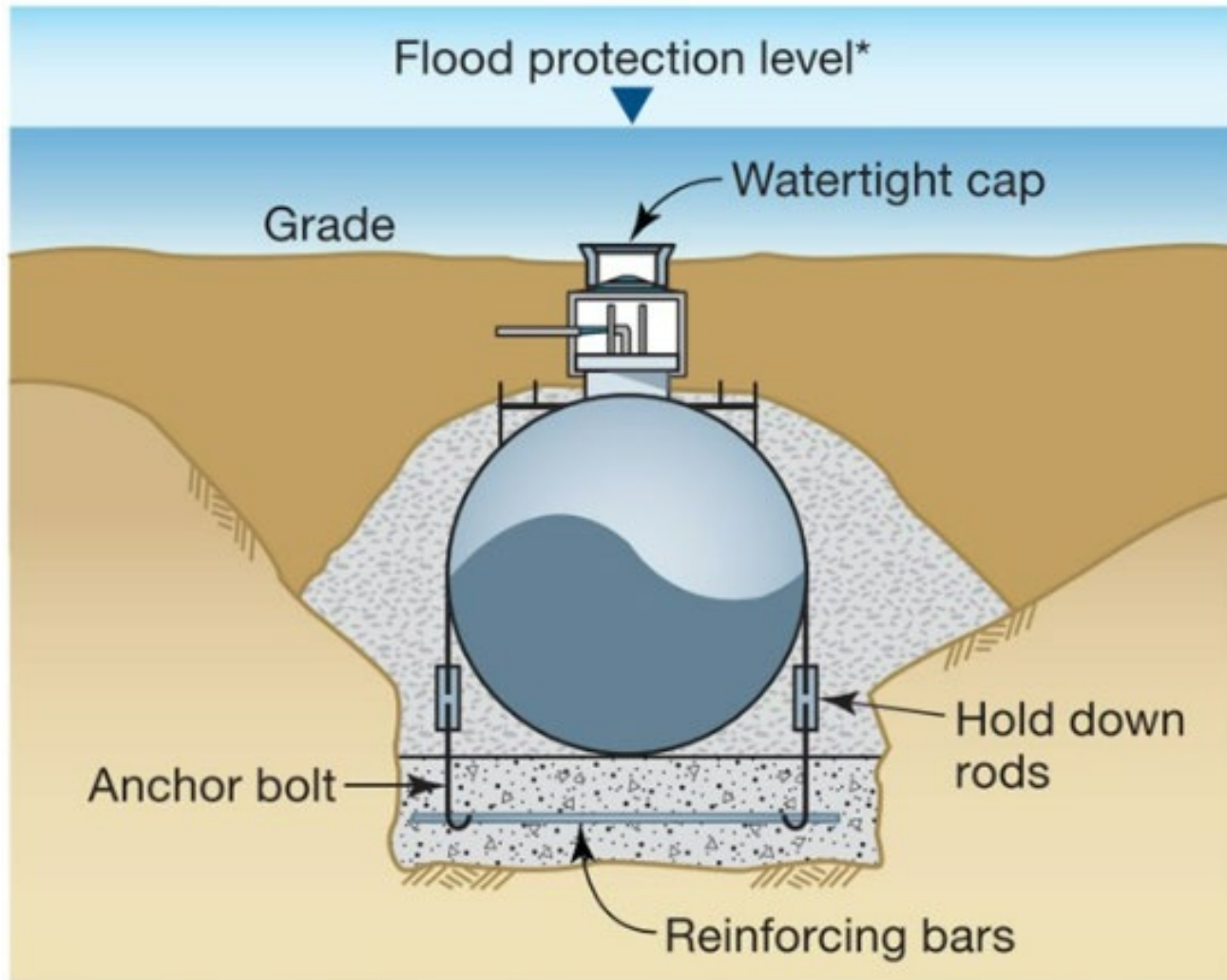
- Questions?



- R18-9-A312.B.4 For on-site wastewater treatment facilities permitted under R18-9-E303 through R18-9-E323, apply the following design requirements, as applicable:
- R18-9-A312.B.4.c Design components, piping, ports, seals, and appurtenances to withstand installation loads, internal and external operational loads, and buoyant forces. Design ports for resistance against movement, and cap or cover openings for protection from damage and entry by rodents, mosquitoes, flies, or other organisms capable of transporting a disease-causing organism;

- Archimedes' principle states that a body immersed in a liquid is subjected to a vertical buoyant force equal to the weight of the displaced liquid.

# Buoyant Forces



\* The flood protection level will be dictated by the ability of the submerged tank to resist depth-dependant hydrostatic pressures

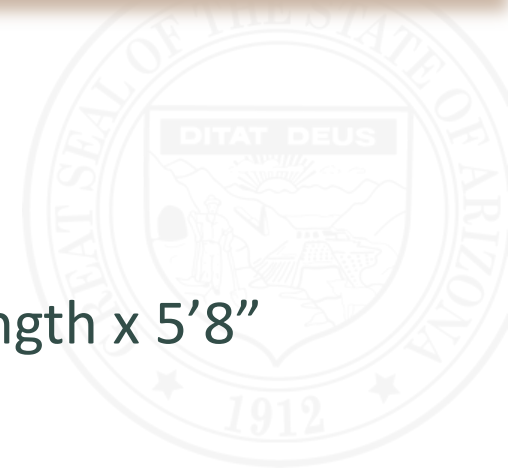
# Buoyant Forces





## – Given

- Septic Tank Volume – 1,000 gallons
- Septic Tank Dimensions – 8'2" (8.17') Length x 5'8" (5.67') Width x 5'1" (5.08') Height
- Soil Cover – 1 foot
- Weight of Concrete – 145 pcf
- Weight of Water – 62.4 pcf
- Weight of Soil – 115 pcf
- Weight of Concrete Anchor – 2,500 #
- Weight of 1,000-gallon concrete septic tank with 3" thick walls – 8,600 #



## – Buoyancy Force

- Buoyancy Force (BF) = Volume of Displaced Liquid x weight of the displaced liquid
- $BF = (8.17' \times 5.67' \times 5.08') \times (62.4 \text{ \#/cf}) = 14,864 \text{ \#}$

## – Resisting Force

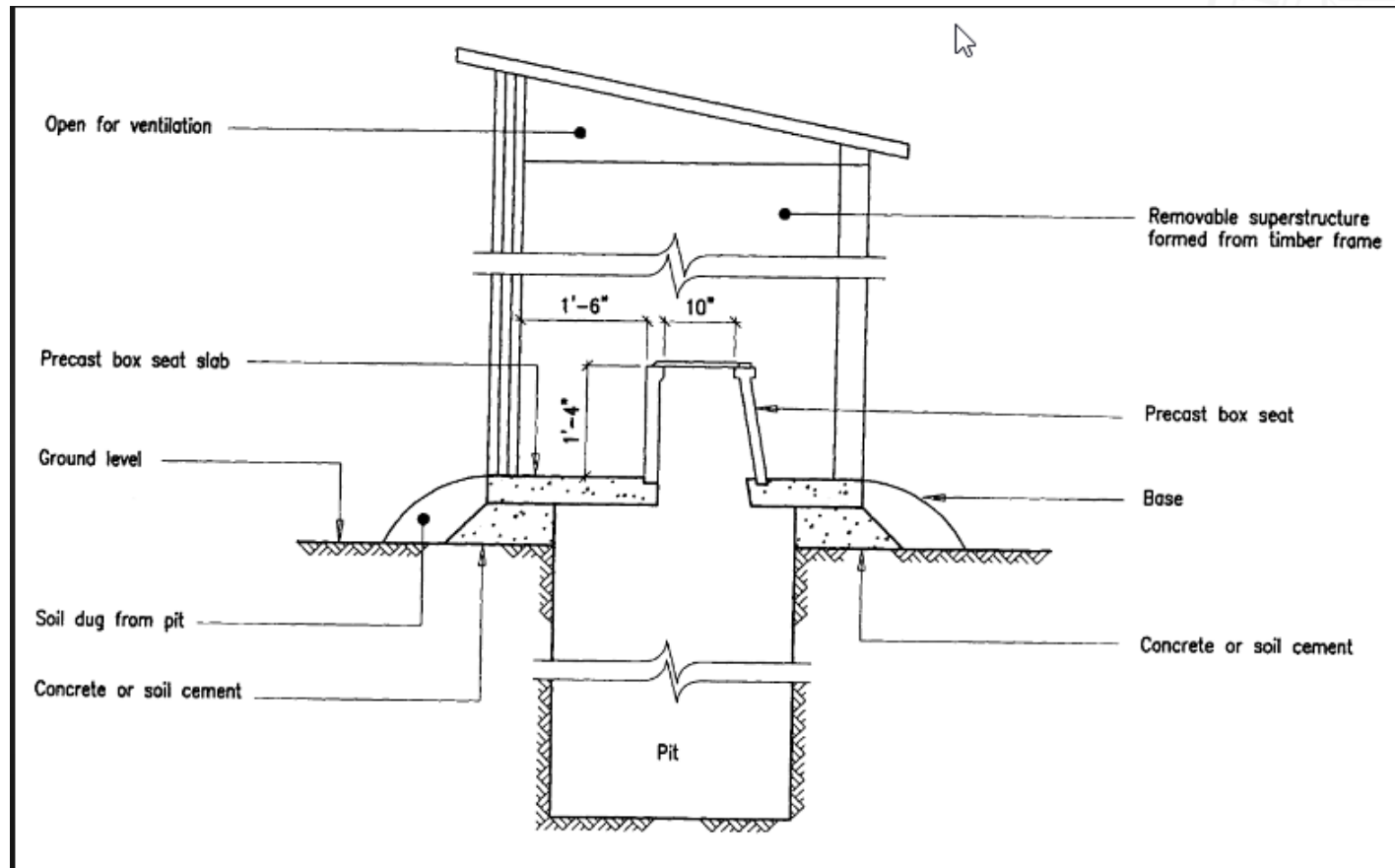
- Resisting Force (RF) = Weight of Concrete + Weight of Soil + Weight of an Anchor
- $RF = 8,600 \text{ \#} + (1' \times 8.17' \times 5.67' \times 115 \text{ pcf}) + (2 \times 2,500 \text{ \#})$
- $RF = 8,600 \text{ \#} + 5,327 \text{ \#} + 5,000 \text{ \#}$
- $RF = 18,927 \text{ \#} > 14,684 \text{ \#}; \text{ the safety ratio} = 1.29$

- Questions?



- R18-9-A309.A.4 A person shall not use a cesspool for **sewage disposal**.
- R18-9-B301.H 1.08 General Permit allows for any **earth pit privy**, fixed or transportable chemical toilet, incinerator toilet or privy, or pail or can-type privy **if allowed by a county health or environmental department** under A.R.S. Title 36 or a delegation agreement under A.R.S. § 49-107.

# Earth Pit Privy vs. Cesspool



- R18-9-101.14 “Cesspool” means a pit, collection structure, or subsurface fluid distribution system, which may or may not be partially lined, that receives discharged sewage. A cesspool is not an on-site wastewater treatment facility, such as a septic tank, vault, or other structure permitted under Article 3 of this Chapter.

- R18-9-101.41 “Sewage” means untreated wastes from toilets, baths, sinks, lavatories, laundries, other plumbing fixtures, and waste pumped from septic tanks in places of human habitation, employment, or recreation. Sewage does not include gray water as defined in A.R.S. § 49-201(20), if the gray water is reused according to 18 A.A.C. 9, Article 7

# Earth Pit Privy vs. Cesspool

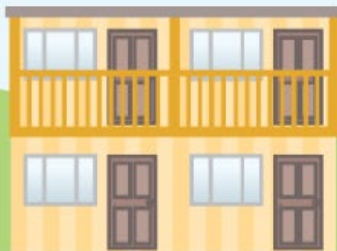


## Large Capacity Cesspools

A Large Capacity Cesspool (LCC) is a hole in the ground that is used to discharge untreated sewage and can serve different building types.

### Multi-Unit Residential LCC

Cesspool serving multiple residential units (e.g., apartment building, duplex, home with mother-in-law unit).



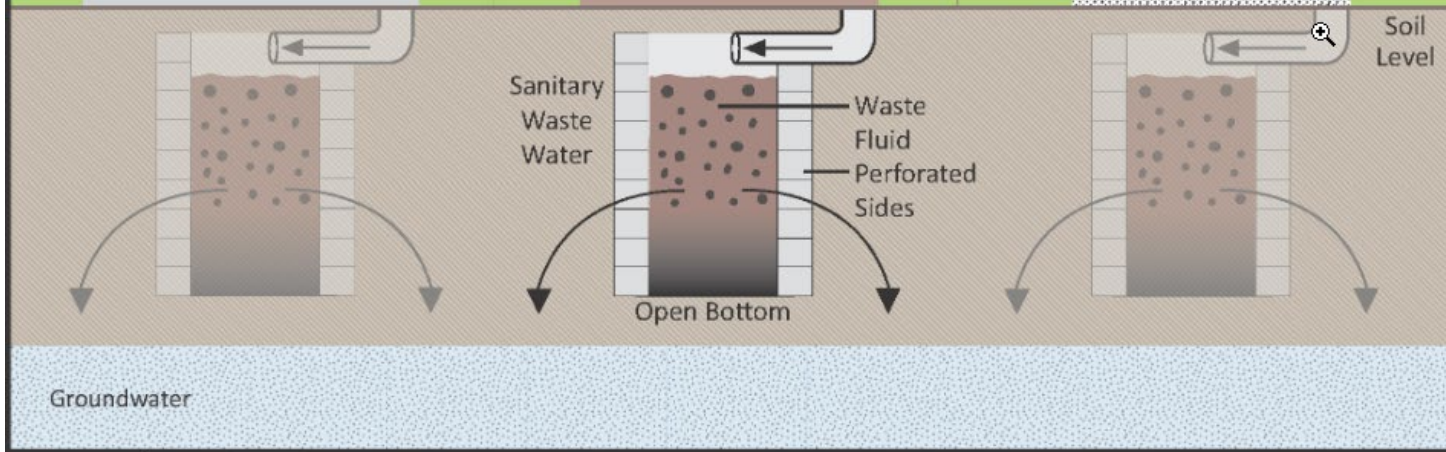
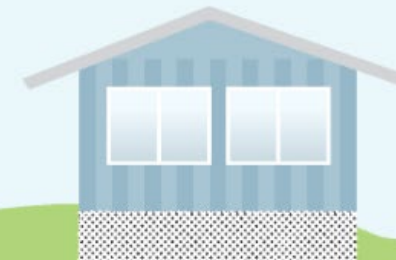
### Non-Residential LCC

Cesspool with the capacity to serve 20 or more people in a given day (e.g., commercial and public service buildings).



### Home Business LCC

Cesspool serving a combined single-family home & a home-based business activity (e.g., beauty salon, childcare center, pet grooming service).







- Analysis of a Concentrated Hydraulic Linear Loading Rate Design
  - Given:
    - Soil – Loamy Sand (SL)
    - Allowable Hydraulic Linear Loading Rate – 5.5 gpd/lf
    - Design Flow - 450 gpd
    - Design Flow per pad = 28 gpd
  - Analysis
    - Zone 1 – 10 pads =  $280 \text{ gpd}/34' = 8.2 \text{ gpd/lf} > 5.5 \text{ gpd/lf}$
    - Zone 2 – 3 pads =  $84 \text{ gpd}/26' = 3.2 \text{ gpd/lf} < 5.5 \text{ gpd/lf}$
    - Zone 3 – 3 pads =  $84 \text{ gpd}/34' + 2.5 \text{ gpd/lf} < 5.5 \text{ gpd/lf}$

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Healthy Land for Everyone**

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